

Leif Hauge, Waukesha County LRD

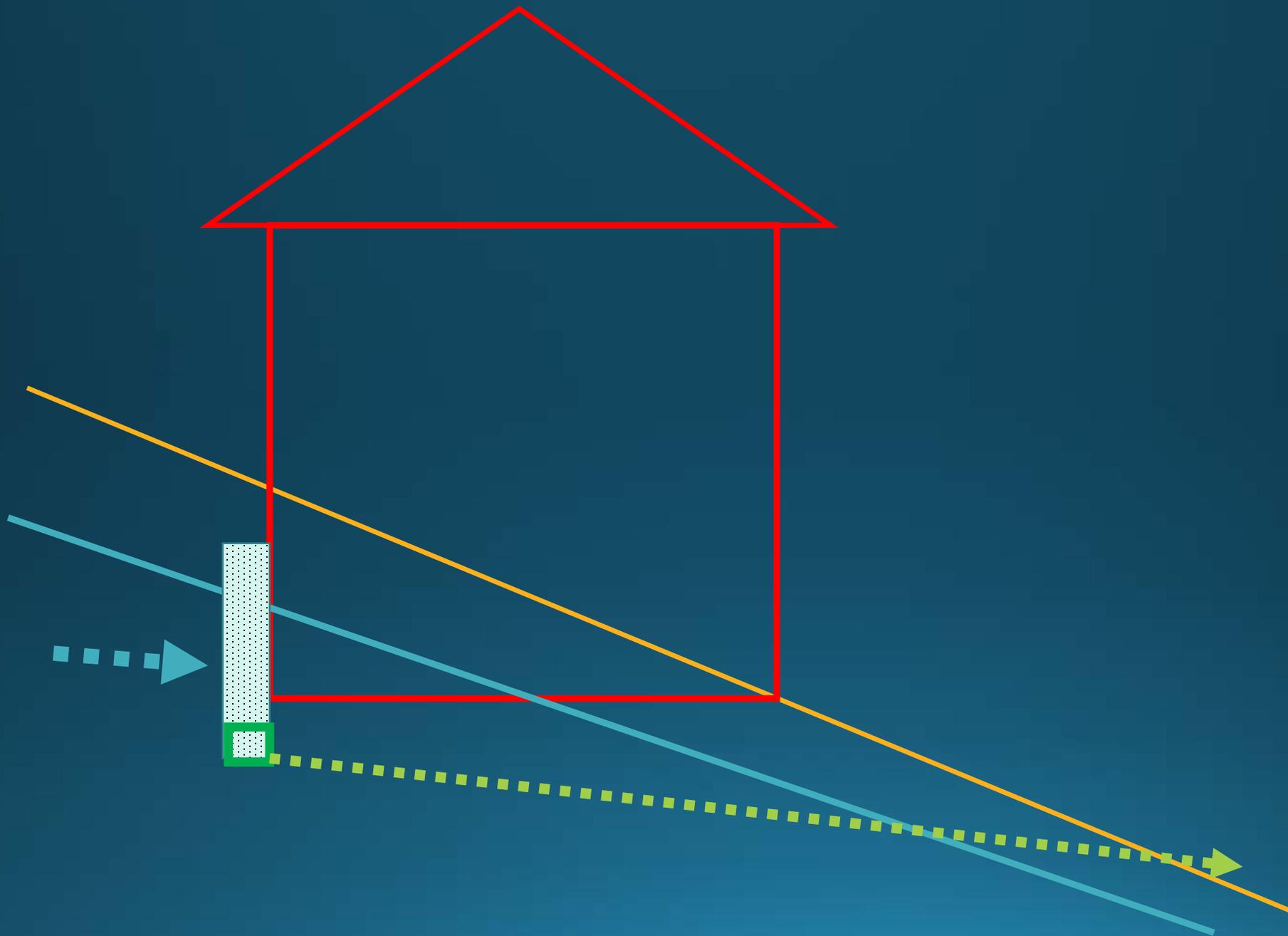
Basement Drainage

Atlas 14

NRCS Soil Class Changes

Basement Drainage

- Ordinance requires 1-foot separation from high water table
- Basement may be allowed in high water table if gravity drainage system can be shown to be protective
- Must show that outside seepage stays within exterior drain tile
- LRD calculation tool is used to estimate seepage flow rates and capacity of drain tile
- Tile may be square, round, sloped and flat



The tool uses

- Dupuit-Fourcheimer Approximation of Darcy's Law to estimate seepage
- Standard Step Method and Newton Raphson Numerical Method (i.e. HEC-RAS technique) to estimate non-uniform gradually-varied flow depth in flat foundation tile
- Manning's Equation to estimate flow depth in sloped outlet pipe

Seepage

Bleeder



Non-uniform

Uniform

Model input and output

- User provides seepage face geometry, hydraulic conductivity, footing and outlet drain tile geometry
- Bleeders connecting inside and outside drain tile are assumed to be 2 inches above bottom of outside drain tile
- Model states whether water in outside drain tile stays below bleeders or not

Water Surface Calculator for Horizontal Formadrain Outletting to Round, Sloped Drain Tile

Inputs by User				Outputs			
Seepage Face Height	2	ft		Seepage Rate	40.00	gal/day, or	0.00006 cfs
Seepage Face Length	100	ft		Flow depth in Drain Tile	<0.005	ft, or	<0.06 inches
Seepage Face Hydraulic Conductivity	10.0000	gal/day/ft ² , see below		Max depth in Formadrain*	<0.021	ft, or	<0.25 inches
Ground or Water Table Slope at seep face	0.02	ft/ft, or	2 percent				
Drain Tile Diameter (4" or 6") *	4	inches, or	0.333 ft				
Elevation difference btwn tile and FAD at joint	0	inches					
Drain Tile Slope	0.05	ft/ft, or	5 percent	Comment:	Formadrain overflows to interior drainage system with bleeder elevation at two inches		
Manning's number for Drain Tile	0.01	dimensionless, 0.010 for PVC					
Formadrain Length	80	ft					

yellow = enter site-specific data

green = outputs

Suggested hydraulic conductivities	
Soil	K (gal/day/ft ²)
Clay, silty clay	0.01
Silt loam, silty clay loam, loam, clay loam, sandy clay loam	10
Sandy loam	100
Gravelly loam, loamy sand	1,000
Sand, fine sand	10,000
Sand and gravel	100,000
Very gravelly sand	1,000,000
Sources: Freeze and Cherry, 1979, p. 29	

* For seepage rates > 8,100 gal/day, exceeds capacity of Formadrain with bleeders at two inches from bottom

Notes and Assumptions

Seepage rate estimated using Dupuit-Forchheimer Approximation of Darcy's Law

No shortcutting by surface runoff directly to drainage system

Flow in sloped, round drain tile is uniform

Manning's equation used to determine flow depth and velocity in drain tile

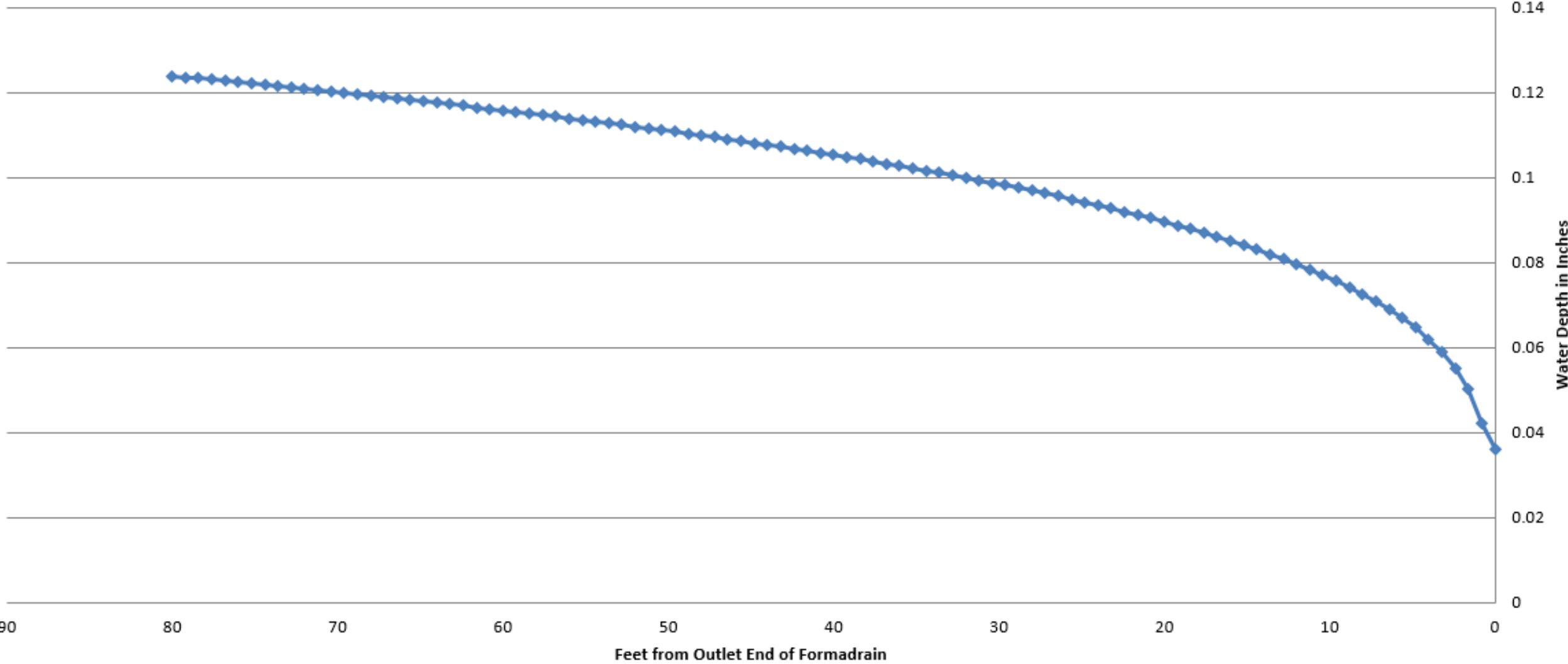
Flow in horizontal FAD is non-uniform and gradually varied

Flow in FAD estimated using Standard Step Method and Newton Raphson Numerical Method

FAD geometry assumed to be 2-inch wide rectangular channel with no bends or top. Bleeders connect inside and outside FAD at 2 inches from bottom of FAD with standard FAD fittings.

No parallel flow in stone around FAD.

Water Depth Profile In Formadrain



Effects Of Switch To NOAA Atlas 14 On Peak Flow Rates

- Higher-intensity rainfall distribution
- Greater depth for 100-year event
- Minor depth changes for some smaller events

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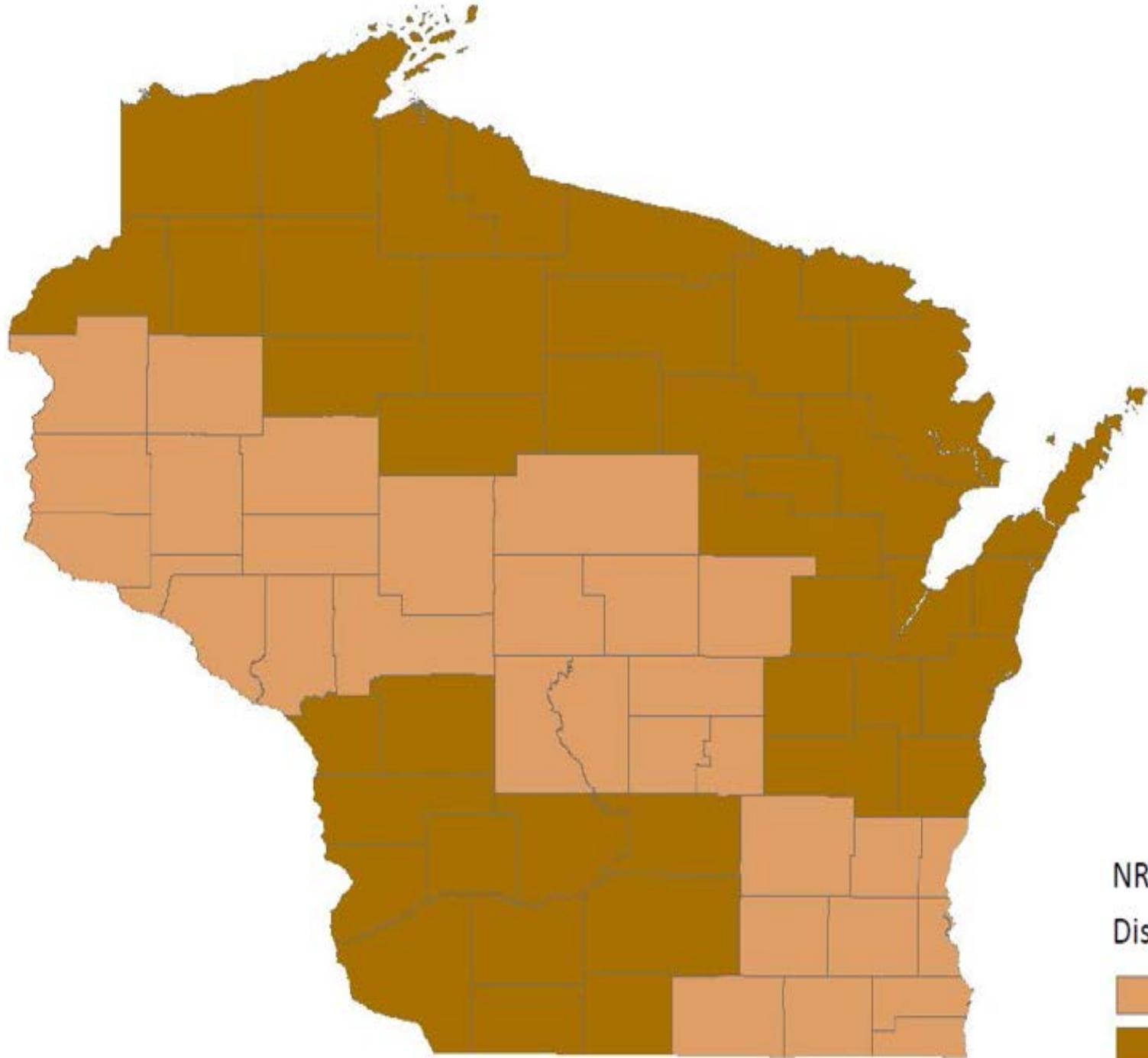
Hydrology, Hydraulics, Climate and Wetland Resources

Notice – The following spreadsheets, computer programs and supporting documentation are provided for informational purposes only. They are not intended to be used for typical hydrologic or hydraulic calculations or wetland hydrologic evaluations. The user must understand the assumptions and calculations contained in these tools and is responsible for their use. Some of the spreadsheets contain macros.

Many additional engineering spreadsheets are available in the [Wisconsin Engineering](#) section.

Hydrology

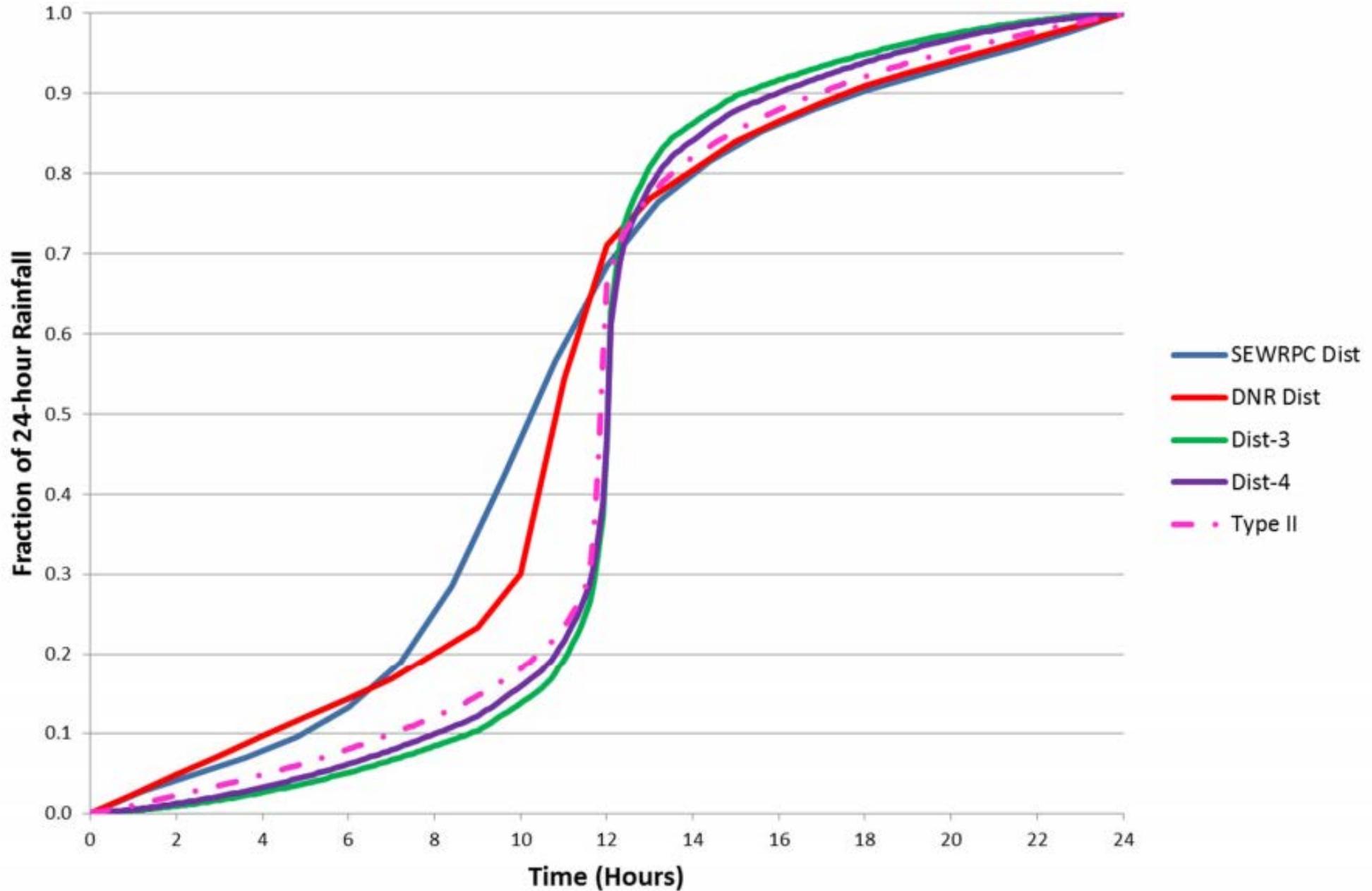
NRCS Wisconsin Hydrology Resources:



NRCS Rainfall
Distribution

- MSE3
- MSE4

Comparison of Storm Distributions



Waukesha County Atlas 14 rainfall depths in inches

	1 year	2 year	10 year	100 year
TP-40	2.3	2.7	4.0	5.6
Atlas 14	2.4	2.7	3.81	6.18
% change	+4	0	-5	+9

- The Atlas 14 100-year rainfall depths range from 5.22 inches in Marinette to 7.78 inches east of Platteville.

Comparison of runoff in cfs from a hypothetical 1 acre site between the new and old rain depths and distributions.

- RCN 70, single watershed
- Tc: 2% slope, 500 feet, 17.3 min.

	1 year	2 year	10 year	100 year
TP 40	0.30	0.52	1.4	2.7
Atlas 14	0.36	0.55	1.4	3.5
% change	+20	+6	0	+31

Comparison of runoff in cfs from a hypothetical 2 acre site between the new and old rain depths and distributions.

- Two 1-acre subwatersheds, both RCN 70,
- Tc: 17.3 min., 30 min.

	1 year	2 year	10 year	100 year
TP 40	0.41	0.74	2.1	4.1
Atlas 14	0.52	0.77	1.9	5.1
% change	+27	+4	-10	+24

Comparison of runoff in cfs from a hypothetical 2 acre site between the new and old rain depths and distributions.

- Two 1-acre subwatersheds, both RCN 70,
- Tc: 12 min., 40 min. (wider spacing)

	1 year	2 year	10 year	100 year
TP 40	0.37	0.66	1.9	3.6
Atlas 14	0.46	0.68	1.7	4.6
% change	+24	+3	-10	+27

Modeled Runoff In cfs From County Highway Reconstruction Project With 138-Acre Total Watershed (Mostly Flow-Through)

	1 year	2 year	10 year	100 year
TP 40	65	84	153	245
Atlas 14	68	82	142	290
% change	+5	-2	-7	+18

- 9 subwatersheds, Tc range 13 – 71 minutes, mostly storm sewer

Possible Conclusions – Atlas 14

- Need 20-30% more storage for 100-year event
- For communities requiring reduction of 100-post to 10-pre, much harder to comply
- Having complex site with multiple subwatersheds with broad range of T_c may reduce change in peak flow
- Slightly larger storm sewers
- Increased setbacks for internally-drained areas

Soil Reclassification by NRCS

B to C

- FmB Fox sandy loam
- MoB Mayville silt loam

B to D

- HmB Hochheim loam

C to B/D

- LmB Lamartine silt loam

Effect on Peak Flow Requirement

- Hypothetical site:
 - Cornfield to office,
 - 4 acres,
 - 70% impervious,
 - Hochheim soil,
 - 2% slopes.
- Pre-development RCN was 69, now will be 83. $T_c = 41$ min.
- Post-development RCN was 87, now will be 92. $T_c = 13$ min (grass ditches).

Peak Flows From Hypothetical Site (cfs)

Condition	1-Year	2-Year	10-Year	100-Year
Pre-developed, B soils	0.66	1.1	2.9	7.8
Post-developed, B soils	5.5	7.1	12	22
Pre-developed, D soils	2.4	3.3	5.9	12
Post-developed, D soils	7.2	8.9	14	23